

Analysis of Mechanical Parameters and Creep Characteristics of Surrounding Rock in Fully Weathered Sandstone Tunnel

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Abstract: Based on the geological conditions of the fully weathered sandstone stratum in the expressway of the mountainous area, the Huizhou-Qingyuan section of the Yuzhan Expressway is used to analyze the mechanical parameters and creep characteristics of the surrounding rock of the tunnel through the indoor model test. It is concluded that: 1) The uniaxial compression deformation test measured the structural parameters of the fully weathered sandstone stratum structure, and obtained that the elastic modulus and internal friction angle ϕ of the fully weathered sandstone was 14.20° , and the cohesion c was 0.343Mpa. 2) The failure mode of fully weathered sandstone specimens was obtained by triaxial creep test. The creep curves of different weathered loads were analyzed and the Xiyuan rheological model was established. 3) The creep data was analyzed by MATLAB software, and the surrounding rock parameters under different surrounding rock were obtained.

1 Introduction

With the number of highways connecting cities has been increasing, and the engineering problems and difficulties is also increasing exponentially. The fully weathered sandstone clay appeared in the Huiqing-Qingyuan section of the Yuzhan Expressway has certain expansibility and disintegration. After the water encounter, the surrounding rock has large deformation and rapid deformation rate. Especially in Guangzhou, the rainwater is rich, which causes a lot of difficulties construction. Therefore, the analysis of the mechanical and creep properties of the surrounding rock of the fully weathered sandstone tunnel is of great significance for selecting appropriate excavation methods, supporting parameters and selecting support timing and reinforcement measures [1-3].

Zhang Haiyang et al. simulated the creep process during excavation of Dazhanggou inclined shaft test tunnel of Muzhailing Tunnel under the condition of high crustal stress soft rock by numerical simulation. The Creep Mechanics Parameters of the geological conditions of the carbonaceous slate were obtained by inversion of the field monitoring data through BP neural network [4]. Liu Mengbo et al. combined with the experimental data of soft clay in Shanghai area, using the creep model coupled with the time hardening rule and Druker-Prager yield criterion, using numerical simulation method to analyze the variation of soft soil creep characteristics and force with time [5].

In this paper, the mechanical parameters of the fully weathered sandstone stratum are measured by laboratory

tests. The creep curves of the specimen under confining pressure are obtained according to the principle of "Chen's loading". The Xiyuan rheological model is established and the creep data is analyzed by MATLAB software. After analysis, the parameters of surrounding rock under different surrounding rock are obtained [6].

2 Project Profile

Through on-site observation and analysis, the Taihedong tunnel has a high content of fully weathered sandstone clay minerals, generally 10 to 30%, and its clay minerals will exhibit certain disintegration and swelling properties, and the water-like properties of the rocks deteriorate seriously. This is also the essential reason for the large deformation of the surrounding rock during the rainy season of the tunnel. The exit section of the Taihedong tunnel reveals that the surrounding rock is fully weathered sandstone.

3 Surrounding rock mechanics parameters

3.1 laboratory apparatus

TAW-200 multi-function rock triaxial test system (Fig. 1), the main parameters of the instrument are as follows: test piece size: $\Phi 50 \times 100$ mm; maximum axial force: 1000kN; maximum ambient pressure: 60MPa; maximum pore pressure: 60MPa; Heating temperature: $-30-80^\circ\text{C}$;

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axial deformation: 0 ~ 20 mm; radial deformation: 0 ~ 5 mm; axial loading speed: 0.01 ~ 1.



Fig. 1 TAW-200 Multifunctional Rock Triaxial Test System

3.2 Experimental program

The uniaxial compression deformation test can measure the elastic modulus E and Poisson's ratio μ of the fully weathered sandstone remolded soil. The size of the uniaxial compression deformation test specimen of the fully weathered sandstone remolded soil is the diameter $\Phi=50\text{mm}$, the height $H=100\text{mm}$. The accuracy of the preparation of the test piece shall not exceed 0.3 mm in diameter over the entire height of the test piece. The non-parallelism of the two end faces does not exceed 0.05 mm at the maximum. The end face shall be perpendicular to the axis of the test piece and the maximum deviation shall not exceed 0.25° .

In order to measure the elastic modulus E and Poisson's ratio μ of the fully weathered sandstone remolded soil, the loading speed was set to 0.01 mm/min, and the confining pressure was 0 MPa, a total of 1*3 test pieces were prepared.

In order to measure the cohesion c and the internal friction angle φ of the fully weathered sandstone remolded soil, confining pressure is 0.4 MPa and 0.6 MPa, 0.8MPa, 1.0MPa and 1.2MPa. a total of 5*3 test pieces were prepared.

In order to obtain the rheological parameters of the fully weathered sandstone remolded soil. The axial load of the test was carried out in a stepwise incremental loading mode, which was divided into five grades (0.6

MPa, 0.8 MPa, 1.0 MPa, 1.2 MPa, 1.4 MPa, 1.6 MPa). A total of 1*3 test pieces were prepared for the test.

3.3 Experimental result

The peak value of the axial stress and axial strain curves is taken as the uniaxial compressive strength, and the uniaxial compression test results of the fully weathered sandstone remolded soil are shown in Table 1.

Table 1 Uniaxial compression test results of fully weathered sandstone remolded soil

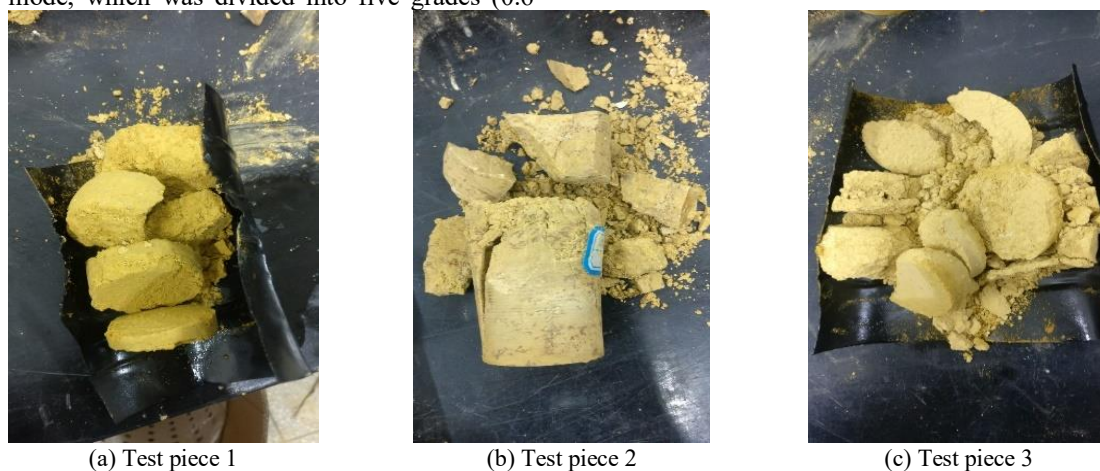
	Axial stress peak S_c / MPa	Secant modulus / MPa	Poisson's ratio
Test piece 1	1.0	23.81	0.32
Test piece 1	0.8	16.33	0.38
Test piece 1	0.5	13.89	0.34
average value	0.77	18.01	0.35

Taking the peak value of the relationship curve between the axial stress and the axial strain as the triaxial compressive strength After averaging the peak intensities measured by the triaxial compression test under five confining pressures, the surrounding stress is plotted on the abscissa according to the Mohr Coulomb strength criterion. The axial stress is the ordinate and the relationship curve when the rock sample is destroyed. By fitting the test data, the slope of the fitted straight line is k , and the intercept with the ordinate is a , and the obtained intensity coefficient k is 1.65, a is 0.88, and the internal friction angle φ is 14.20° , and the cohesion is obtained. The force c is 0.343 MPa.

4 Creep characteristics

4.1 Triaxial creep test results

The fully weathered sandstone remolded soil typical of the triaxial creep test is shown in Fig. 2.



(a) Test piece 1

(b) Test piece 2

(c) Test piece 3

Fig.2 Fully weathered sandstone remolded soil specimen after triaxial compression creep damage

4.2 Analysis of test results

At the time of loading, it should be noted that the time interval of the step-by-step loading should be equal, and the lower-level loading must be performed after the deformation of the pre-stage load has entered the steady-state creep. According to the principle of "Chen's loading" method, the creep curve under different loads can be obtained. Due to the low strength of the original rock specimens and the looser particles, the test results of the specimens are more discrete. The number of grades of test piece 1 and test piece 2 is small, and the test result of test piece 3 is selected for analysis, as shown in Fig. 3.

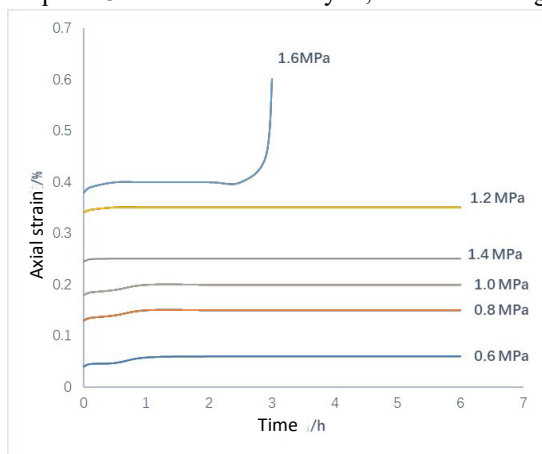


Fig.3 Creep curve of specimen 3 under different loads

4.3 Analysis of creep characteristics

According to the characteristics of the creep curve obtained by the experiment, the comparative analysis shows that the Nishihara rheological model is more suitable for simulating its rheological characteristics. Under the triaxial stress, the nonlinear viscoelastic-plastic creep constitutive equation of rock is (Xiyuan model):

$$\begin{cases} \varepsilon = \frac{\sigma_1 + 2\sigma_3}{9K} + \frac{\sigma_1 - \sigma_3}{3G_0} + \frac{\sigma_1 - \sigma_3}{3G_1} \left(1 - e^{-\frac{G_1 t}{\eta_1}}\right), \sigma_1 \leq \sigma_s \\ \varepsilon = \frac{\sigma_1 + 2\sigma_3}{9K} + \frac{\sigma_1 - \sigma_3}{3G_0} + \frac{\sigma_1 - \sigma_3}{3G_1} \left(1 - e^{-\frac{G_1 t}{\eta_1}}\right) + \frac{\sigma_1 - \sigma_s}{2\eta_2} t, \sigma_1 > \sigma_s \end{cases}$$

K — Bulk modulus

G_0 — Instantaneous shear modulus

G_1 — Shear modulus

h_1, h_2 — Viscosity coefficient

The parameters in the above formula are identified by MATLAB. The surrounding rock parameters under different surrounding rock are optimized as shown in Table 2.

Table 2. Triaxial compression creep model parameter identification results

σ /MPa	K /MPa	G_0 /MPa	G_1 /MPa	h_1 /MPa · h	h_2 /MPa · h	R^2
0.6MPa	10.67	12.93	527.60	487.95	—	0.98851
0.8MPa	7.91	17.13	356.91	286.62	—	0.993767
1.0MPa	7.28	19.08	260.03	262.31	—	0.982397
1.2MPa	6.88	20.74	204.31	220.12	—	0.983596
1.4MPa	6.61	22.03	170.65	190.25	—	0.994435
1.6MPa	6.81	23.72	179.34	25.32	18.65	0.995565

5 Conclusion

In this chapter, through the indoor test and numerical limit analysis of surrounding rock mechanical properties, the surrounding rock stability, instability failure mode and monitoring and early warning standards of the fully weathered sandstone section of Taihe tunnel are studied, and the following conclusions are obtained.

(1) The mechanical parameters of the strongly weathered argillaceous sandstone were obtained through laboratory tests. The internal friction angle φ was 14.20° and the cohesion c was 0.343Mpa.

(2) Through the triaxial creep test, different confining pressures are applied to the fully weathered sandstone. According to the principle of the Chen loading method, the creep curves under different loads can be obtained, and the suitable conditions for the surrounding rock are

obtained. Rheological model, and the creep model parameters are obtained through MATLAB software analysis.

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