

Stability analysis of concrete block retaining wall based on a scaled laboratory

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Abstract. A concrete block retaining wall is supported by its weight. This block retaining wall structure has some advantages, including lower construction costs, a water-permeable construction that produces less water pressure behind the wall, and a more flexible construction because it can follow the ground's contours. Rankine's theory is usually used to design this block retaining wall. According to this theory, the failure pattern behind the wall forms an angle of $45^\circ + \phi/2$ with the horizontal plane. The laboratory tests indicate that the pattern of failure or the pattern of sand movement behind the wall is similar to the letter S. From the sandy soil of failure pattern curve, look for a functional equation approximating the pattern. The equation of the function obtained is an equation of the third-order function. An analysis of the sliding, overturning, and overall stability block retaining wall is based on this equation of the cube function. Analysis for overall stability using the method of slices, dividing the failure area by several slices. These function equation order three is needed to get the area and length failure.

1 Introduction

The purpose of retaining walls is to support the weight of sloped soil and buildings, earthquake loads, machine loads that produce vibrations, and others [1]. It is important to note that retaining walls can remain functional and stable based on the weight of the retaining wall itself and the weight of the soil above the heel of the wall.

The advantages of a concrete block retaining wall include its ease of construction; the footing is simply a gravel leveling pad, and the units are dry-stacked without the use of mortar, steel reinforcement, or grouting [2], [3]. The Concrete block retaining walls are often considered as rigid (conventional) retaining walls when designing shear and overturning stability [4–6].

The failure behavior or pattern of a block retaining wall and a gravity wall is not necessarily the same on the field. In laboratory experiments, the failure plane of sand behind the concrete block retaining wall is not a straight line, as predicted by Rankine's theory (a straight line), but is more like an S curve [7].

There have been several failures when designing retaining walls. Sharma (2011) reported on a study on the failure of retaining walls, which serves as a helpful lesson for future consideration of the stability of retaining walls [8]. The situation was discussed in detail in the report, including the retaining wall's failure mechanism. The failure of retaining walls is mainly caused by the incorrect

design of retention and support systems based on experience [9-10].

As a result, a more thorough examination of this retaining wall is required, beginning with examining the failure pattern.

2 Block Retaining Wall (Concrete)

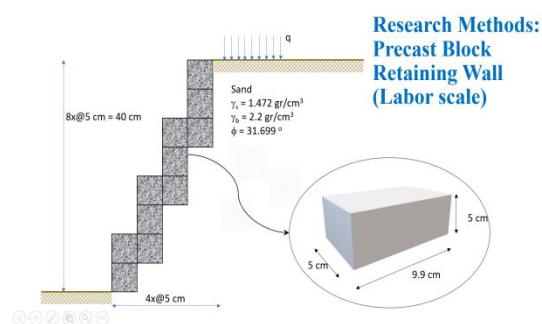


Fig. 2. Failure pattern due to static load (Rankine's theory)

In the laboratory, the model of the block retaining wall was made, as shown in Fig. 1. The block retaining wall is made of a mixture of cement, sand, and water with 9.9 (length) x 5 cm (width) x 5 cm (height). The blocks are

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arranged in a glass box with a diameter of 40 cm (length), 10 cm (width), 80 cm (height). The surface of the embankment behind the wall is level, with the soil type being sand. The total height of the wall is 40 cm, and there are 11 blocks in total. There is no adhesive or connection between the concrete blocks.

In classic theory (Rankine's theory), to calculate the overall stability of the block (segmental) retaining wall, assuming the failure pattern behind the wall resembles a straight line forming an angle of $(45^\circ + \phi/2)$ with the horizontal plane in Fig. 2. According to NCMA, the block retaining wall failure pattern is predicted to appear like a straight line in Fig. 3. even though the actual on the field is not always the same [11].

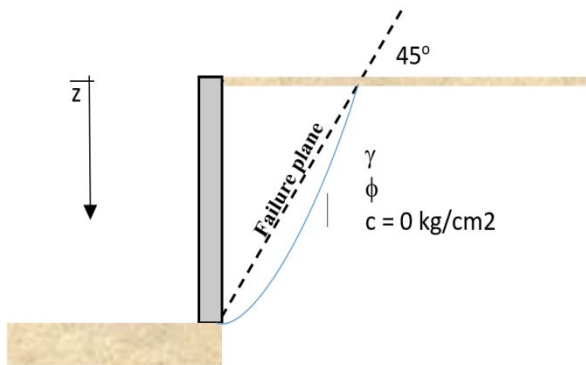


Fig. 2. Failure pattern due to static load (Rankine's theory)

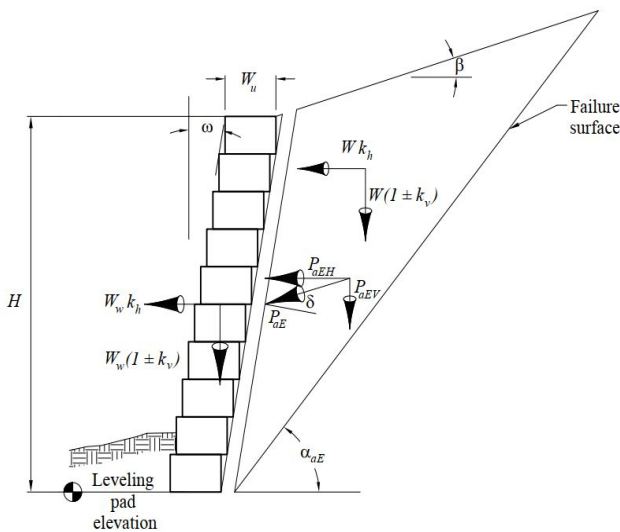


Fig. 3. NCMA, design SRW (2016)

3 Method

The static load is given gradually until the wall fails after the block retaining wall is arranged, as indicated in Fig. 1. During the testing process, videos are recorded, so the movement of sand grains can be seen. The next step is to determine the equation function of the failure pattern. This equation's function helps determine the wall's overall stability, as well as the backfill area and length. Analysis for overall stability using the slices method applies to flat, circular, or a combination of both.

With the method of slices, the soil block that has failed is divided into several slices. Vertical slicing is used to divide the slices. For each piece, the working forces are analyzed and calculated cumulatively. Furthermore, the safety factor of the assumed failure plane can be calculated by comparing the resisting and driving forces. Weight is determined by the soil's weight for each part is:

$$W = \gamma (\Delta A) \quad (1)$$

Next, the normal force (N) and the tangential force (T) act for each slice (α is the slope angle assumed for each slice):

$$N = W \cos \alpha \quad (2)$$

$$T = W \sin \alpha \quad (3)$$

The area of the failure (A) is calculated using the integral equation of $F(h)$ (Equation 4). $F(h)$ is a function of the equation for the height of the block retaining wall. The formula determines a slice's failure length (DL) in equation 5. By using equation 6, we can calculate the overall stability safety factor ($c = 0$).

$$\text{area of slices } A (\Delta A) = \int_a^b F(h) \quad (4)$$

$$\Delta L = \int_a^b \sqrt{1 + \left[\frac{dF(h)}{dh}\right]^2} dh \quad (5)$$

$$SF_{\text{overall}} = \frac{\sum_1^n (c + \Delta L_n + N_n \tan \phi)}{\sum_1^n T_n} = \frac{\sum_1^n (N_n \tan \phi)}{\sum_1^n T_n} \quad (6)$$

A safety factor of sliding and overturning is also investigated in this study. The force of lateral earth pressure, the weight of soil above the heel (W_s), and the concrete W_c should all be considered while analyzing the wall's stability. An active earth pressure condition will arise if the wall is pushed out due to the earth pressure behind it. According to Rankine, the value of earth pressure can be calculated as follows:

$$\sigma_a = \gamma z \tan^2 \left(45^\circ - \phi/2\right) \quad (7)$$

Using the active earth pressure coefficient, K_a , as a starting point:

$$K_a = \tan^2 \left(45^\circ - \phi/2\right) \quad (8)$$

Only the weight of the soil itself contributes to the active compressive force acting behind the wall to a depth of $z = h$ in non-cohesive soils ($c = 0$).

$$P_a = 1/2 \gamma h^2 K_a \quad (9)$$

Equation 10 is the formula for determining the value of the safety factor of sliding. The formula for the safety factor of overturning is explained in equation 11.

$$SF_{\text{sliding}} = \frac{\sum F_R}{\sum F_d} = \frac{\sum V \tan \delta + Bc + P_p}{P_a} = \frac{\sum V \tan \delta}{P_a} \quad (10)$$

$$SF_{\text{overturning}} = \frac{\sum M_R}{P_a y} = \frac{\sum W x}{P_a y} \quad (11)$$

4 Result and Discussion

Table 1 shows the data on soil properties. Because the cohesion value is equal to 0 and the sieve analysis reveals 96.096% sand, the soil type is sand. Based on observations and video recordings of laboratory experiments, the movement pattern of sand grains was obtained Fig. 4. The pattern is described in the Cartesian coordinate system Fig. 5. This pattern (failure pattern) looks like the letter S.

Table 1. Sandy soil properties

Type of Parameter	Value
Unit Weight (γ_s)	1,472 gram/cm ³
Soil Cohesion (c)	0 gram/cm ²
The angle of internal friction of soil (ϕ)	31,699°
Sieve analysis of soil	96,067 % (sand)

The next step is to find the equation of the mathematical function of the pattern Fig. 6. The function obtained is the equation of polynomial order 3, which is shown in equation 12. An analysis of overall stability is performed based on the failure pattern.



Fig. 4. NCMA, design SRW (2016)

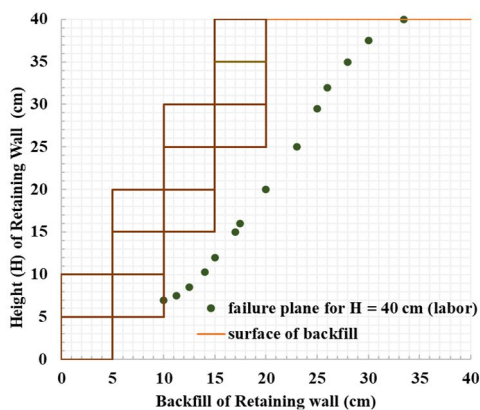


Fig. 5. Sketch of failure pattern

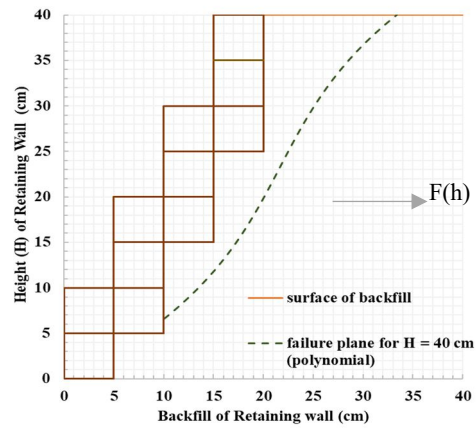


Fig. 6. The mathematical function of the failure

$$F(H) = 0.0008H^3 - 0.0558H^2 + 1.7739H + 0.5262 \quad (12)$$

4.1 Overall Stability of Block Retaining Wall

Based on the equation of function, the overall stability of the wall can be found. Fig. 7. shows the failure area divided into seven slices using the method of slices.

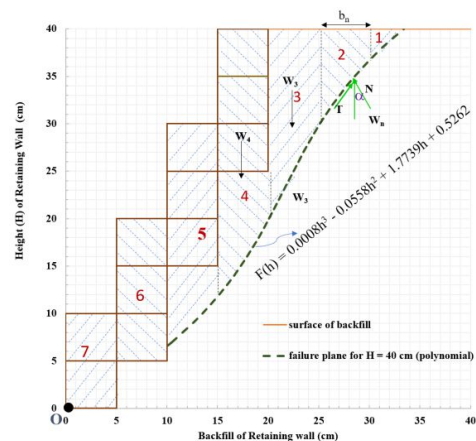


Fig. 7. Equation of failure pattern and method of slices

The area and weight of each slice are calculated using equations 1 dan 4. Additionally, the calculation is displayed in a tabular format following:

Table 2. Calculation of overall stability

Slice	b	Area	γ	Weight	α	$\sin \alpha$	$\cos \alpha$	$W \sin \alpha$	$W \cos \alpha$
	cm	cm ²	gr/cm ³	gr/cm ³	°			gr/cm ³	gr/cm ³
1 sand	3.40	5.42	1.47	7.98	46.00	0.72	0.69	5.74	5.55
2 sand	5.00	32.52	1.47	47.86	57.00	0.84	0.54	40.14	26.07
3 sand	5.00	75.33	1.47	110.89	62.00	0.88	0.47	97.91	52.06
4 sand	5.00	47.47	1.47	69.88	52.00	0.79	0.62	185.09	144.61
5 block	5.00	75.00	2.20	165.00					
6 block	5.00	75.00	2.20	165.00	0.00	0.00	1.00	0.00	165.00
7 block	5.00	50.00	2.20	110.00	0.00	0.00	1.00	0.00	110.00
							Sum	293.02	653.61

$$SF_{overall} = \frac{\sum_1^n (N_n \tan \phi)}{\sum_1^n T_n} = 1.16 < 1.5 \quad (13)$$

The overall stability safety factor value is 1.16, based on the preceding explanation of the slice approach (equation 6).

4.2 Sliding Stability and Overturning Stability

Fig. 2 depicts the forces involved in estimating sliding and overturning stability. The lateral earth pressure force P_a , the weight (W) of the soil and concrete, and the force's distance (x or y) from the point O (toe of a wall) are all factors to consider. Table 3 summarizes the calculations used to determine the sliding stability value.

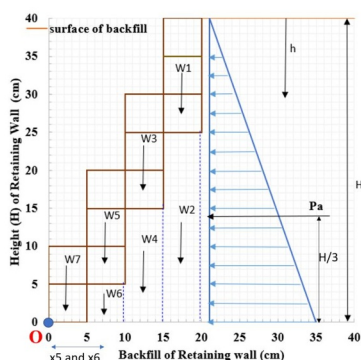


Fig.8. Forces acting on the block retaining wall

Tabel 3. Sliding stability calculation

Slice	Area cm ²	γ gr/cm ³	W gr/cm ³	Distance cm	Moment gr/cm ³
1	75	2.2	165	17.5	2887.50
2	125	1.47	183.75	17.5	3215.63
3	75	2.2	165	12.5	2062.50
4	75	1.47	110.25	12.5	1378.13
5	75	2.2	165	7.5	1237.50
6	25	1.47	36.75	7.5	275.63
7	75	2.2	165	2.5	412.50
Total		$\Sigma V =$	990.75	$\Sigma M_R =$	11469.38
		$P_a =$	365.74	13.33	4875.26
		$K_a =$	0.311		

The safety factor value for sliding stability is:

$$SF_{sliding} = \frac{\Sigma V \tan \delta}{P_a} = \frac{990.75 \cdot \tan(\frac{2}{3} + 31.699)}{365.74} = 1.05 < 1.5 \quad (14)$$

The safety factor value for overturning stability is:

$$SF_{overturning} = \frac{\Sigma W x}{P_a y} = \frac{11469.38}{4875.26} = 2.35 < 2.5 \quad (15)$$

5 Conclusion

The failure or movement of sand grains behind the block retaining wall generates an equation of a polynomial order of three functions. Order three of the function equation can be used to calculate the overall stability value. Using

the integral of the equation of the power of three functions, determine the area of failure. So that calculations using these equations will get more precise results. It is necessary to redesign the block retaining wall dimensions to obtain a safe overall, sliding, and overturning stability value.

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